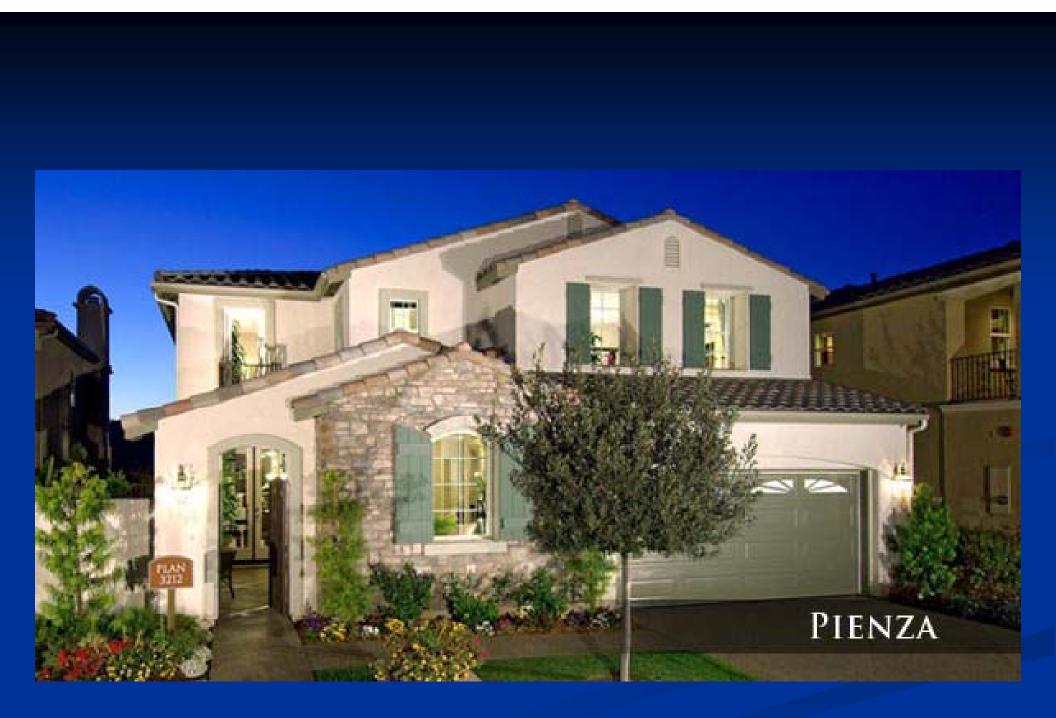
STRUCTURAL SYSTEM OF WOOD BUILDING IN AMERICA

What is American Style Wood House?

Majority of houses in US is "SINGLE FAMILY RESIDENCE"













































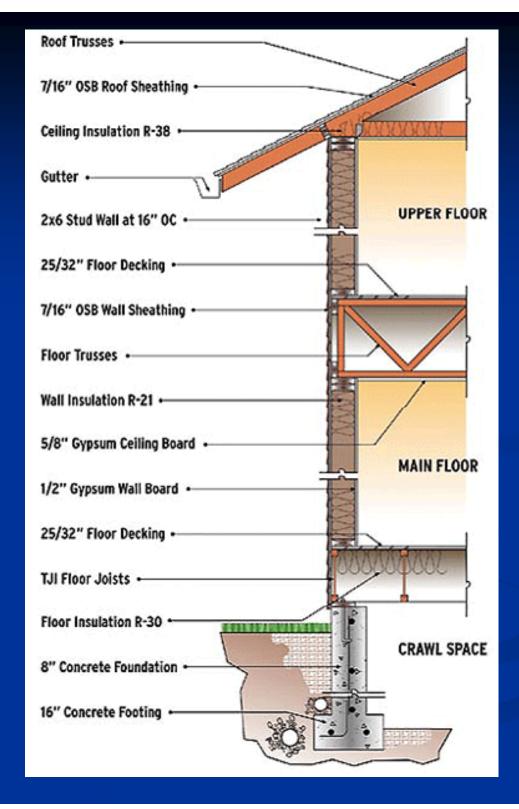
 The majority of residential buildings such as Single Family Residence, Town Home, Town House, Duplex, Multiplex, Apartment, Condos in US are constructed with Wood Framing enclosed by

(1) Stucco, stone veneer over exterior face of walls and by gypsum board on interior face;

(2) Concrete tile or composite shingle on top of inclined roof and with gypsum board at underside of roof ceiling;

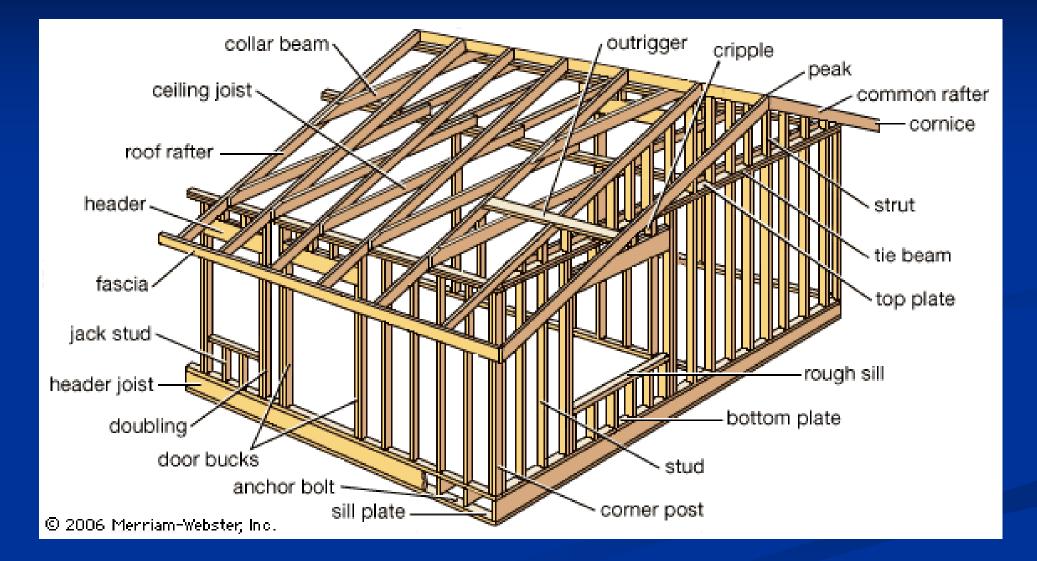
(3) Hardwood or tile or stone on top of floor and with gypsum board at underside of floor ceiling;

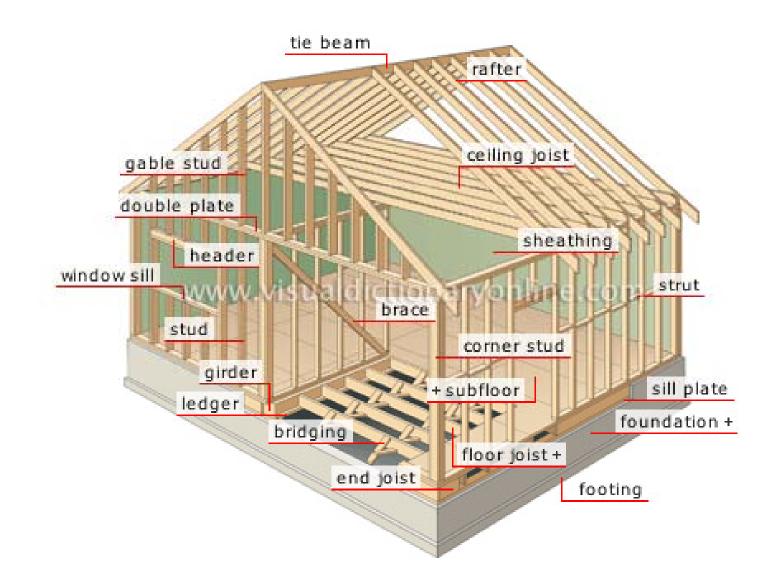
(4) Concrete slab on grade or raised wood floor with continuous strap concrete footing along the perimeter and interior concrete pad footing

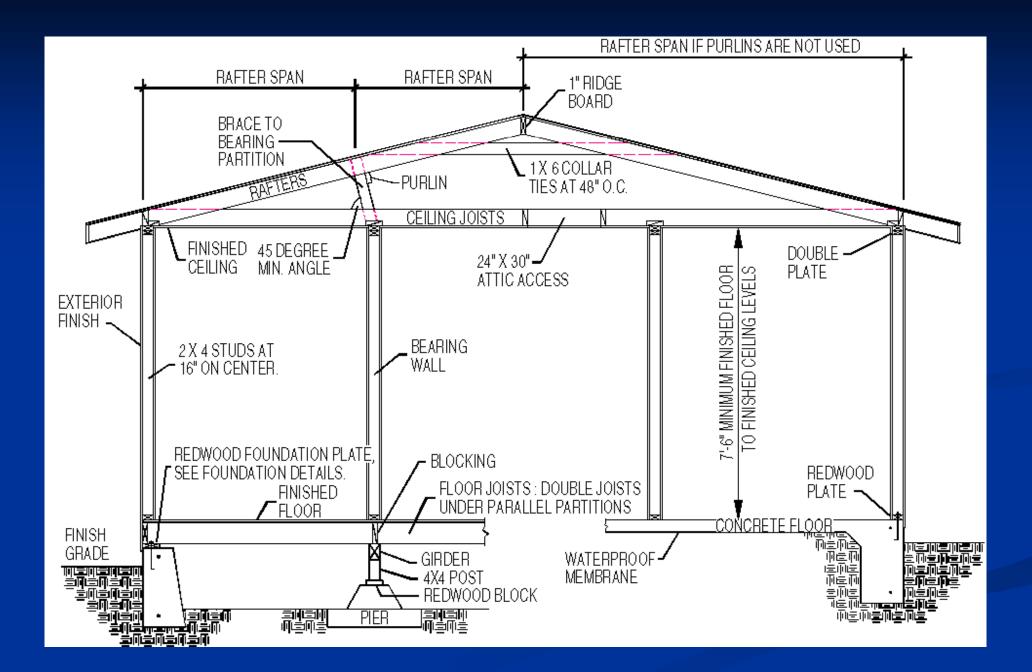


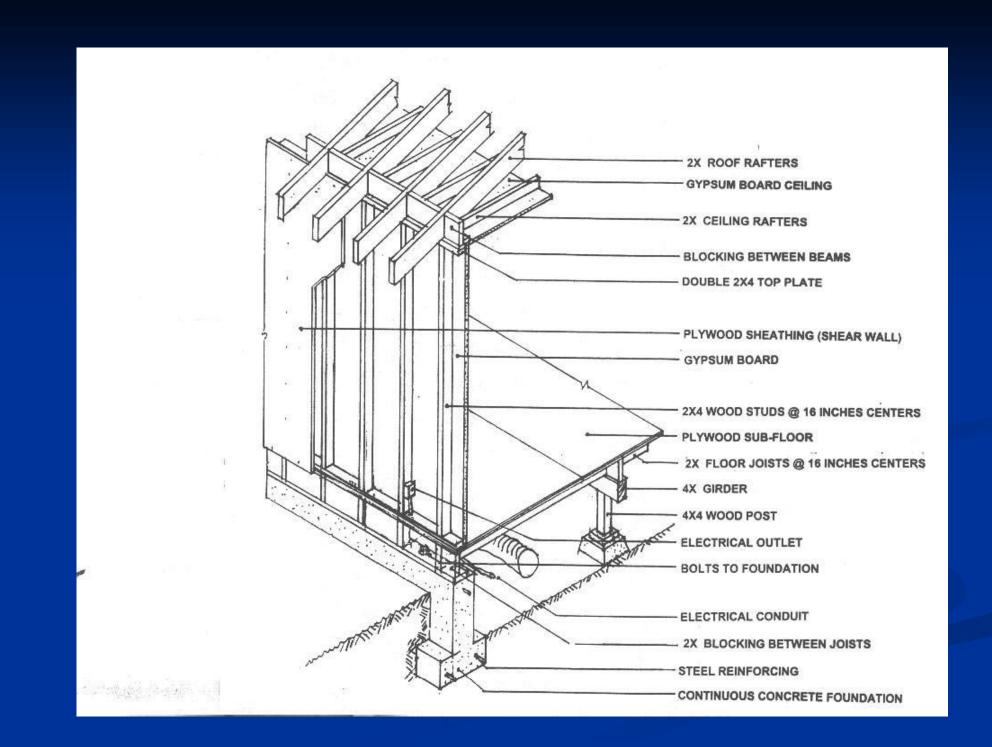
ANATOMY

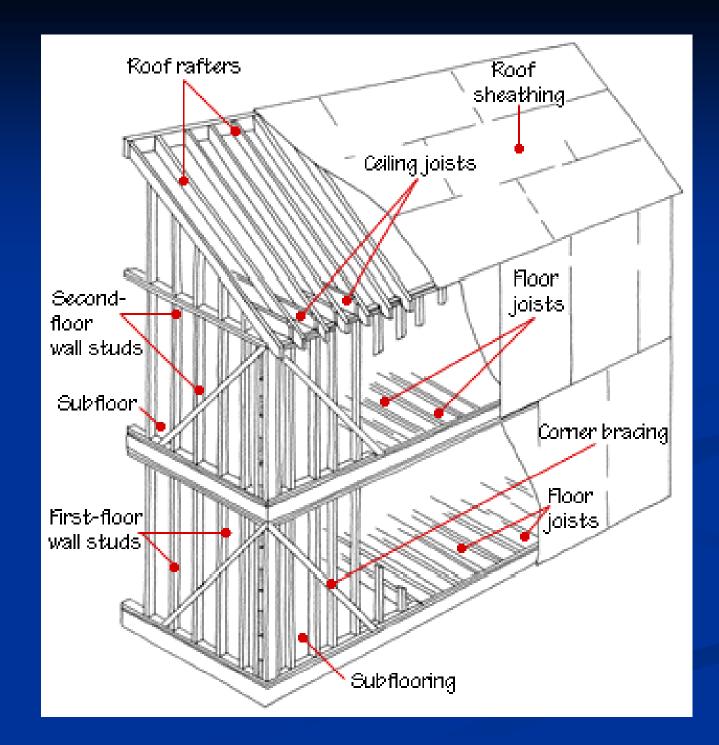
Wood Framing (1) **Roof** (b) Rafter (c) Ridge (a) Diaphragm (d) Ceiling Joist (e) Kicker & Purlin (2) **Floor** (a) Floor Joist (b) Floor Beam (3) Wall (b)Stud (a) Double Top Plates (c)Sole plate Concrete Foundation & Basement Wall

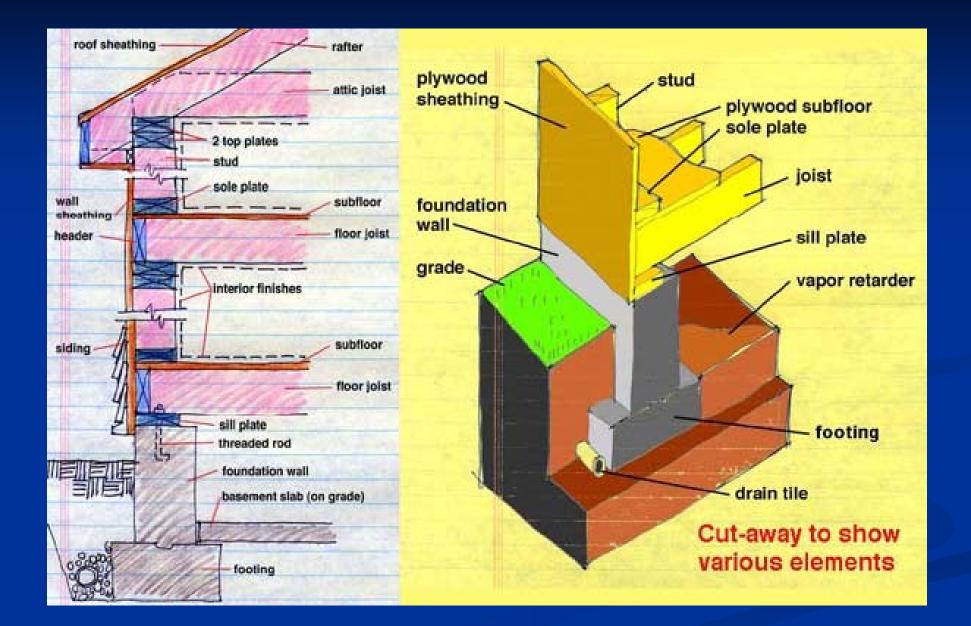


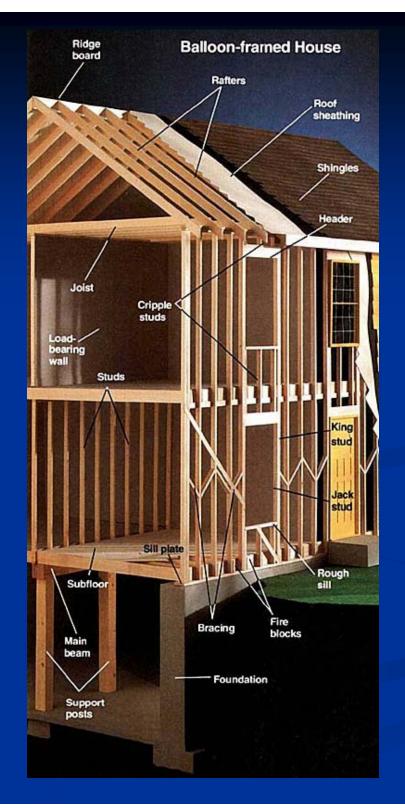
































Advantages of Using Wood as Structural Members

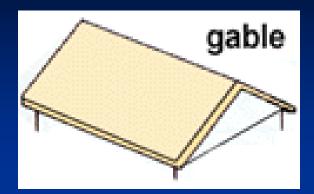
- High Ratio of Strength to Weight
- Low Earthquake-Induced Inertia Load
- High Energy Absorption (for Seismic and/or Wind)
- Low Cost per Pound
- Good Electrical Insulation, Low Thermal Conductance
- Warmth, Beauty, Versatility, Durability, Workability
- Wide Ranges of Finishes Can be Used
- Wide Ranges of Wood Species Can be Used
- Reusable, Recyclable, Green, and more

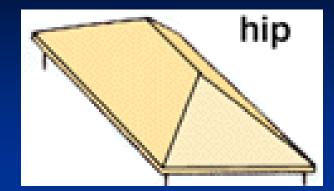
Disadvantages of Wood Frame Construction

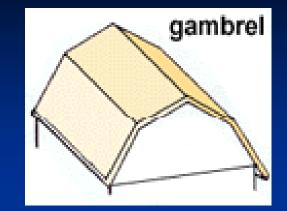
- Sound Noise
- Fire Resistivity
- Moisture Control
- Non-Homogeneous, Not Isotropic, Orthotropic
 (3 axis)
- Variation of Strength Under Different Situations
 Wood Degrading Organisms (Biodeterioration): Fungi (cause molds), Insects (termites), Bacteria, Marine Borers

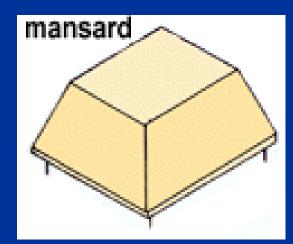
Roof System

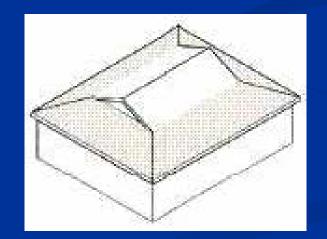
- Gable Roof (2 rafters & one ceiling joist)
- Hipped Roof (2 rafters & one ceiling inclined)
- Dutch Roof (Gable-on-Hip)
- Clipped Roof (Hip-on-Gale)
- Sloped Roof
- Others such as Flat Roof, Gambrel Roof, Mansard Roof

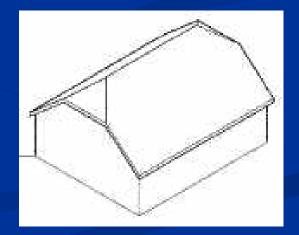












How to Construct Roof

- Triangle (Flat Ceiling)

 (1) Home-Made Trusses (2) Pre-manufactured Trusses

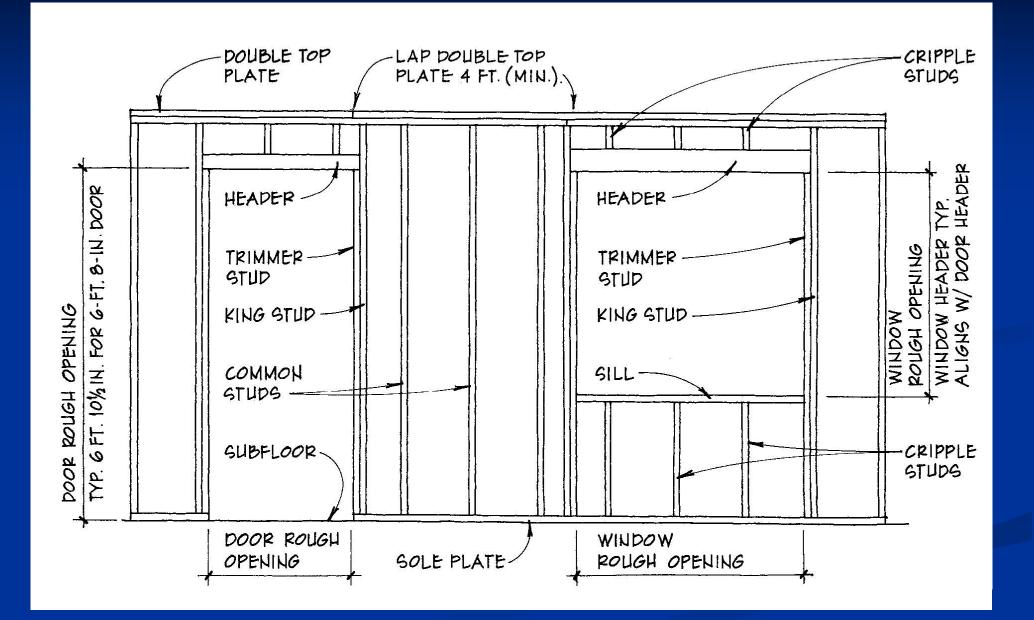
 Vaulted Ceiling

 Inverted V, Letter A, Kicker & Purlin, Ridge Beam & Post
- Recessed Ceiling
- Scissor Roof

Wall Frame System

The wall is constructed with
(1) Double Top Plats (2) 2x Studs @16" O.C.
(3) Sole Plat (4) With or Without Structural Panel

(5) The Gypsum Board on the Interior Side and the Exterior Weather Cover such as Stucco, Siding Usually Are Not Considered Structurally



Wood Frame Wall

Shear Wall
Non-Shear Wall
(1) Bearing Wall
(2) Non-Bearing Wall

How to make a frame wall to be Shear Wall ??? To be Bearing Wall ??? To be Non-Bearing Wall ???

Floor Frame System

FLOOR DIAPHRAGM

- ³/₄" THK STRUCT II PLYWOOD T&G GPI, 5 PLY 48/24 GLUE W/ 10d@6" O.C. ALL EDGES & 10" O.C. IN FIELD
- 2x10, 2x12, 2x14 FL JOISTS@16" O.C.
- PRE-MANUFACTURED FL JOISTS
 - TJI, TJL, TJW, TJS, TJM, TJH, OPEN-WEB JOISTS, OTHERS
- FLOOR BEAMS

SWAN LUMBER 4x10, 4x12, 4x14

ENGINEERING TIMBER: GL BM, MICRO LAM, VERSALAM, PARALLAM, POWERBEAM, ETC.

Ground Raised Floor

- 2x6 FL JOISTS@16" O.C. W/ 7'-6" SPAN
- 4x8 FL W / 7'-6" SPAN SUPPOR TING 2x6S
- 4x4 POST SUPPORTING 4x8S
- 18" TO 24" x12" THK CONC. PAD FTGFL DIAPHRAGM

³/₄" THK STRUCT II PLYWOOD T&G GPI, 5 PLY 48/24 GLUE W/ 10d@6" O.C. ALL EDGES & 10" O.C. IN FIELD

Foundation

On Flat to Minor Slope Ground
 (1) Continuous Strap Inverted T shape Footing along Perimeter
 (2) Interior Pad Footing or Slab On Grade

 On Hill Slope
 (1) Continuous Grade Beam With Piers along Perimeter and Interior

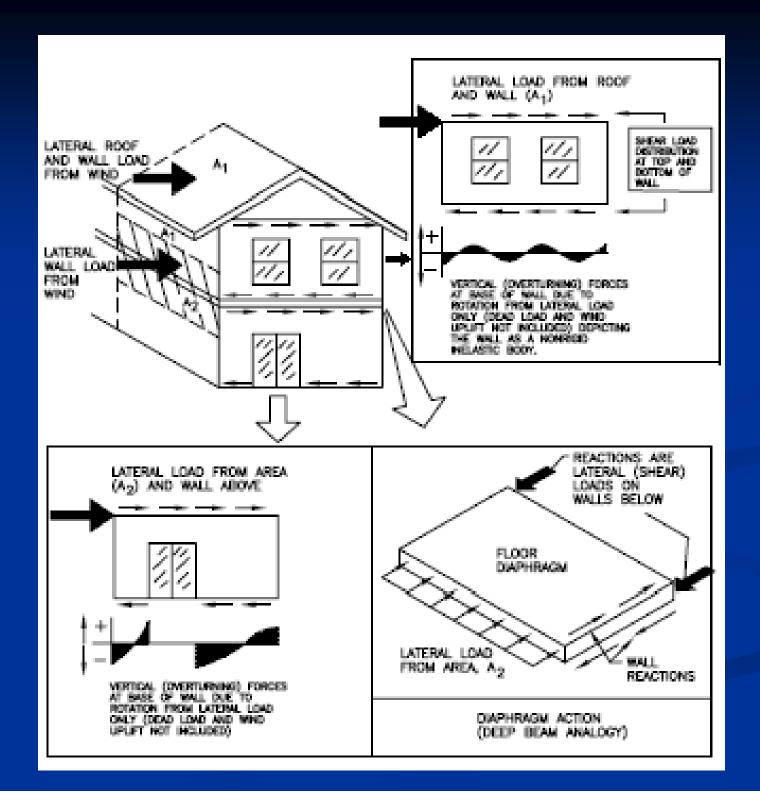
LATERAL FORCE RESISTING SYSTEM

HORIZONTAL DIAPHRAGM SHEAR WALL

A continuous load path should be designed to carry the lateral force due to natural loadings such as earthquake and wind from the roof and floors down to the foundations

Continuous Load Path

- A continuous load path should be designed to carry the lateral forces due to natural loadings such as earthquake and wind from the roof and floors down to the foundations to safeguard the life of residents
- To ensure this all the members, elements, and connections should be designed with adequate strength and stiffness to achieve above purpose



Basic Concepts

Box Idea:

Treat the wood building as a pseudo box composed of (1) Horizontal diaphragm (roof or floor wood platform laid horizontally), and

(2) Vertical diaphragm (Wood shear wall per code, Simpson Strong Wall, Simpson Steel Strong Wall, Hardy Frame, TJI Shear Wall, Cantilevel free standing column, Steel moment frame, Simpson Moment Frame)

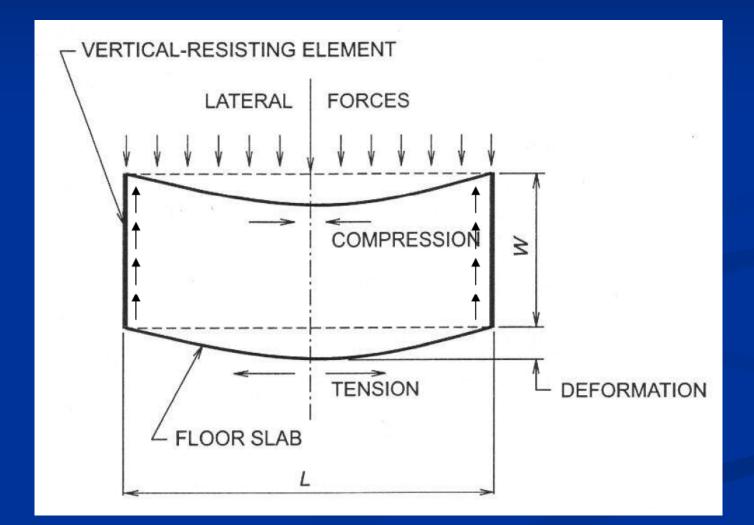
(2) Collectors (drag strut) in line with Vertical Lateral Elements

(3) Base foundation of concrete and rebars

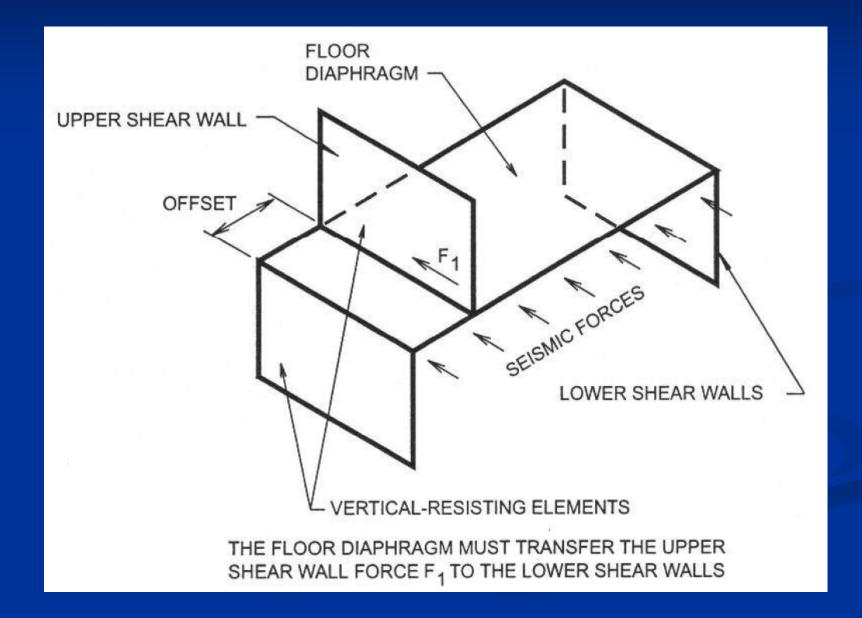
Horizontal Diaphragm

- Roof framing clad with structural panels with specified nailing schedule
 - 1/2" PLYWOOD STRUCT II W/ 10d NAILS @6"O.C. ALL SUPPORTED EDGES, 12" O.C. IN FIELD LAYOUT THE LONG DIMENSION OF DIAPHRAGM PERPENDICULAR TO FRAMING MEMBER BEL
- Floor framing clad with structural panels with specified nailing schedule
 - 3/4" PLYWOOD STRUCT II T&G GP I, 5 PLY 48/24 GLUE W/ 10d NAILS @6" O.C. ALL SUPPORTED EDGES, 10" O.C. IN FIELD

Horizontal Diaphragm



Horizontal Diaphragm

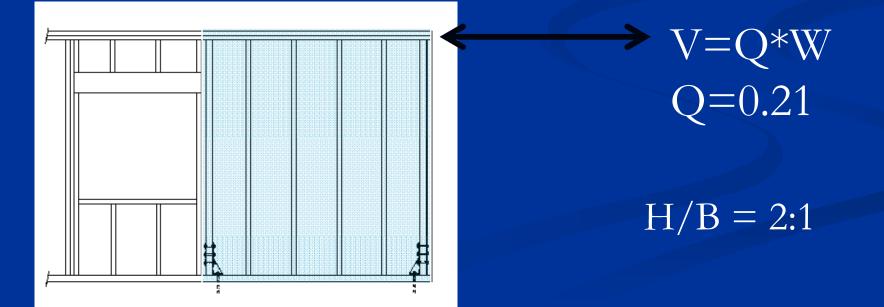


Shear Wall

Ordinary Wood Shear Wall Perforated Wood Shear Wall Wood Shear Wall with Opening reinforcement Engineering Shear Wall (1) Simpson's Strong Wall (2) Simpson's Steel Strong Wall (3) Hardy Frame (4) Others

Ordinary Wood Shear Wall

Wood framing wall with structural panel on one or both side with specified nailing per code to resisting lateral force and with or without holddowns to resisting overturning



Ordinary Wood Shear Wall

2x4 (or 2x6)@16" O.C. W/ double top plates on top and sole plate (and sill plate) at bottom W/ ½" thk Struct II plywood W/10d @6" O.C. all edges and 12" O.C. in field

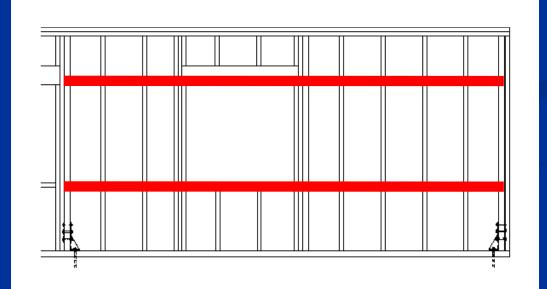
SHEAR WALL TYPE

TYPE	DESCRIPTION	SILL PLATE & ANCHOR BOLTS	SOLE PLATE NAILING	CLIP ANGLE © TOP PLATE	SHEAR CAPACITY
	1/2" STR II PLYWD W/ 10d @6" O.C. @PNL EDGES, 12" O.C. INFIELD	W/ 5/8"A.B.	16d @ 6"O.C.	LS50 @ 16"O.C.	310 #/FT
	1/2" STR II PLYWD W/ 10d @4" O.C. @PNL EDGES, 12" O.C. INFIELD	W/ 5/8"A.B.	SDS 1/4"X3" WOOD SCREW @ 8" O.C.	LS50 @ 12" O.C.	460 #/FT
3	1/2" STR II PLYWD W/ 10d @3" O.C. @PNL EDGES, 12" O.C. INFIELD	W/ 5/8"A.B.	SDS 1/4"X3" WOOD SCREW @ 6" O.C.	LS50 @ 12" O.C.	600 #/FT
	1/2" STR II PLYWD W/ 10d @2" O.C. @PNL EDGES, 12" O.C. INFIELD	W/ 5/8"A.B.	SDS 1/4"X3" WOOD SCREW @ 4" O.C.	2-LS50 @ 16" O.C.	700 #/FT

Wood Shear Wall with Opening Reinforcement

- For shear walls with openings, where framing members, blocking, and connections around the openings are designed for force transfer around the openings (force-transfer shear walls) the following provisions shall apply.
- Design for force transfer shall be based on a rational analysis.

Wood Shear Wall with Opening Reinforcement

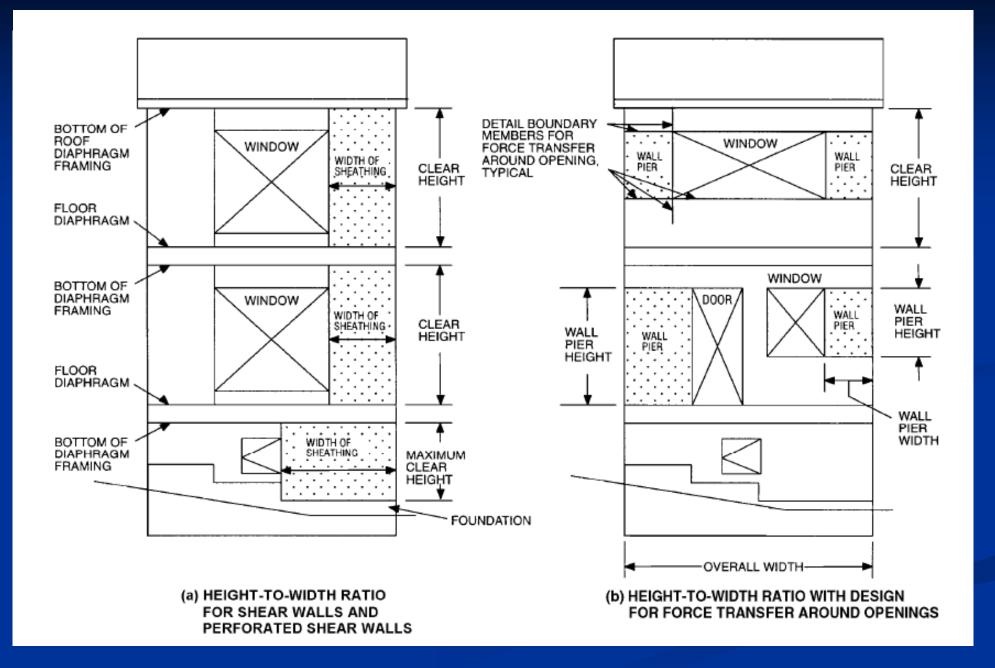


2CS14, MST60

HDU2, HDU5, HDU6

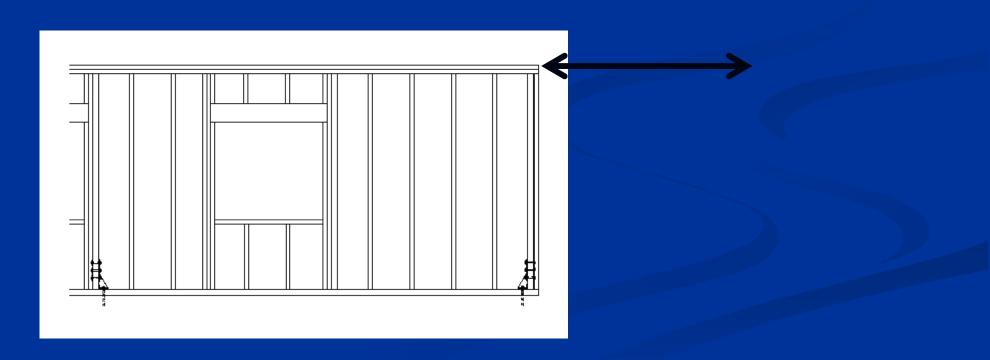
Wood Shear Wall with Opening Reinforcement

- The following limitations shall apply:
- 1. The length of each wall pier shall not be less than 2'.
- 2. A full-height wall segment shall be located at each end of a force-transfer shear wall.
- 3. Where out-of-plane offsets occur, portions of the wall on each side of the offset shall be considered as separate force-transfer shear walls.
- 4. Collectors for shear transfer shall be provided through the full length of the force-transfer shear wall.



Perforated Wood Shear Wall

Where wood structural panel shear walls with openings are not designed for force transfer around the openings, they shall be designed as perforated shear walls.



Perforated Wood Shear Wall

Where wood structural panel shear walls with openings are not designed for force transfer around the openings, they shall be designed as perforated shear walls.

$$v = \frac{V}{C_o \Sigma L_i}$$
(Equation 23-4)

where:

- v = Unit shear force, pounds per lineal feet (N/m).
- V = Shear force in perforated shear wall, pounds (N).
- C_o = Shear resistance adjustment factor from Table 2305.3.8.2.
- $\sum L_i$ = Sum of widths of perforated shear wall segments, feet (mm).

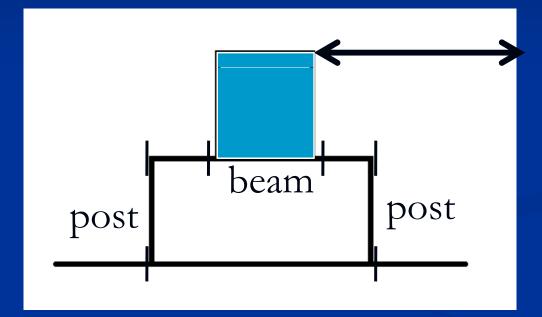
Shear Load Path of Shear Wall

Two story Building

(1) Shear Wall above 2nd floor supported by beam and shear transfer through 2nd floor diaphragm

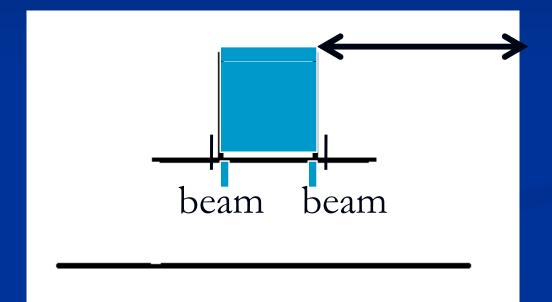
(2) Shear Wall above 2nd floor supported by floor joisted and blockings, and shear transfer through 2nd floor diaphragm

Shear Wall Supported by Beam



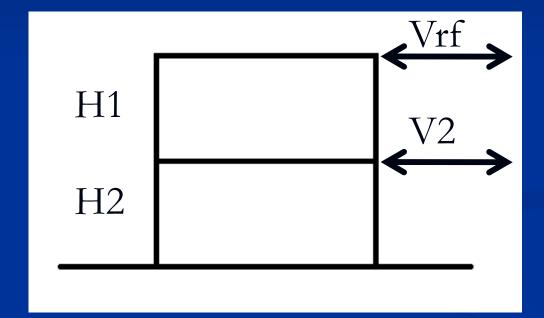
Special Load Combination for design the shear wall supported by the floor beam due to the C & T

Shear Wall Supported by Beam

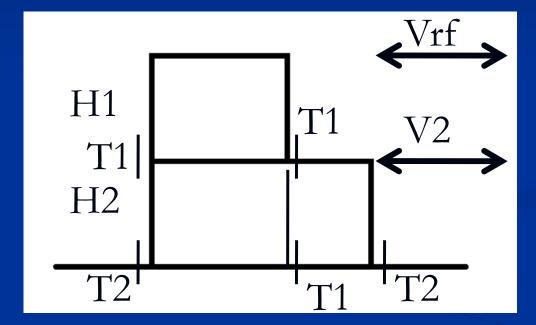


Special Load Combination for design the shear wall supported by the floor beam due to the C & T

- Two story Building
 - (3) Stack up two story Shear Wall with same length
 - (4) Stack up two story Shear Wall with different length

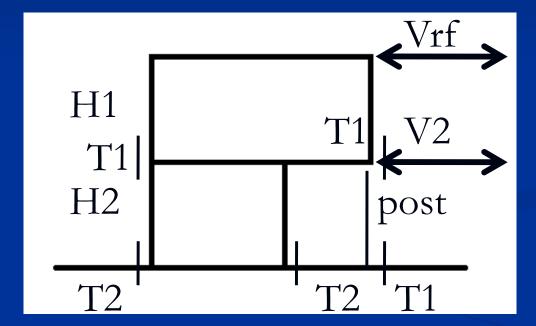


T & C @ 2nd FL = Vrf*H1/L
T & C @ 1st or Ground FL = (Vrf*(H1+H2)+V2*H2)/L



• T & C (a) 2^{nd} FL T1=C1=Vrf*H1/L1

- T & C @ 1st or Ground FL T2 = Vrf*H1/L1+(Vrf+V2)*H2/L2

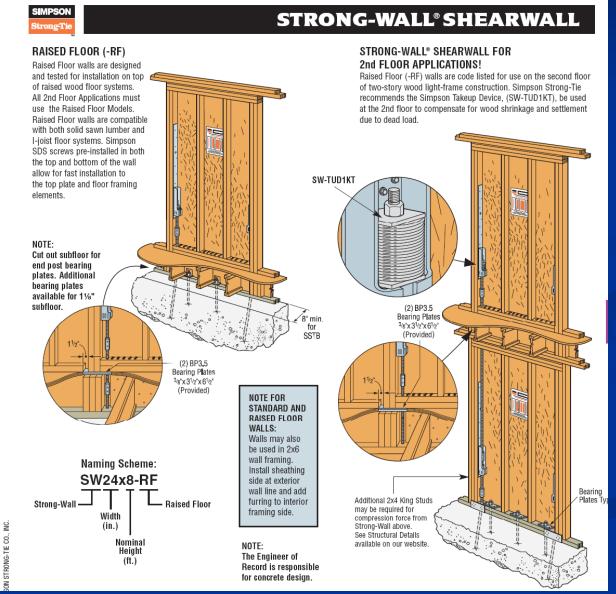


T & C @ 2nd FL = Vrf*H1/L1
T & C @ 1st or Ground FL = Vrf*H1/L1+(Vrf+V2)*H2/L2

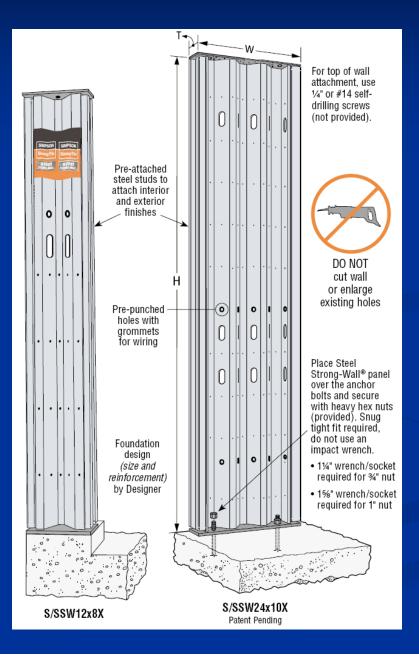
Engineering Shear Wall

- Simpson Strong Wall
- Simpson Steel Strong Wall
- Hardy Frame
- Others (other manufacturers)

Simpson Strong-Wall



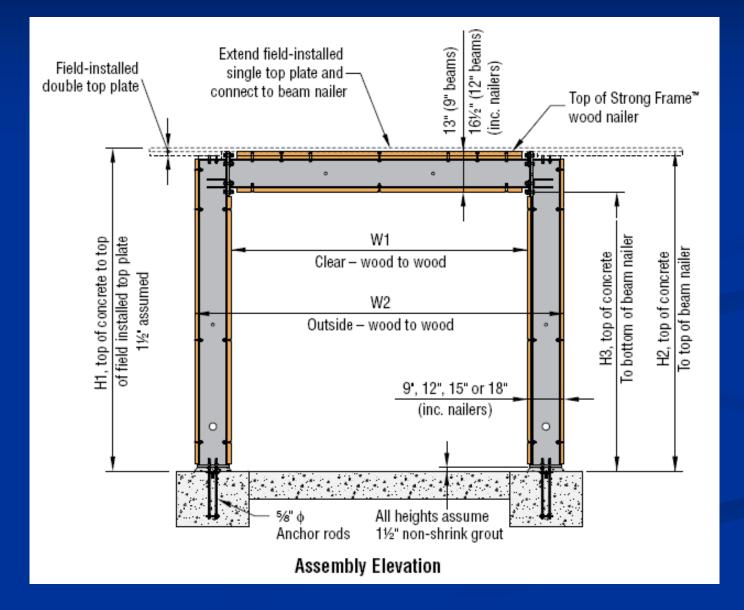
Simpson Steel Strong Wall



Steel Moment Frame

- Ordinary Steel Moment Frame
- Simpson Moment Frame
- Not used in Wood buildings as possible unless necessary

Simpson Strong Frame

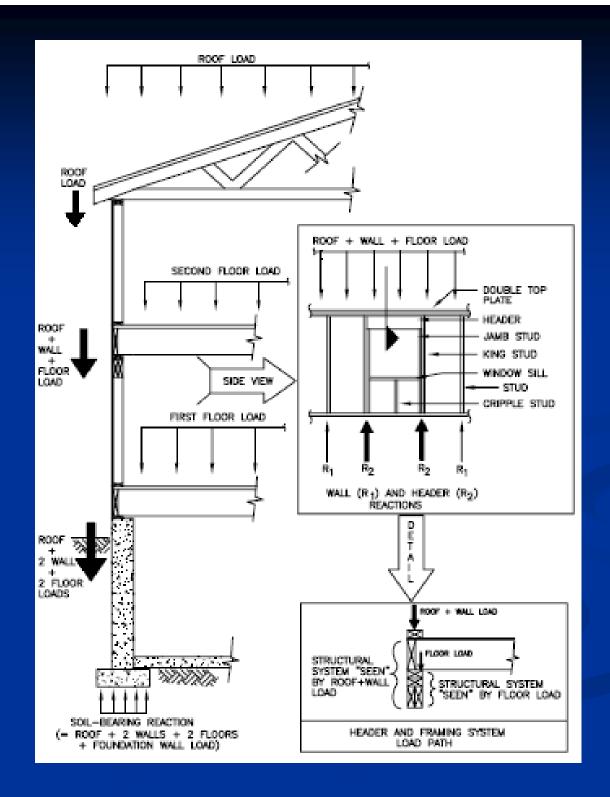


Cantilevel Column

Wood Cantilevel Column
Steel Cantilevel Column
TS Tube , Pipe, etc.
Not used in Wood Buildings as possible as necessary

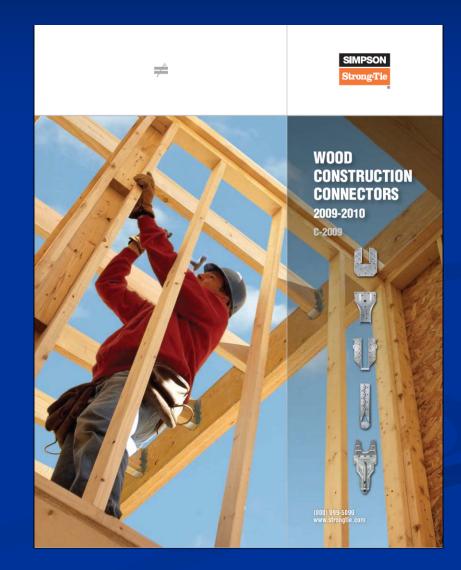
GRAVITY LOAD SYSTEM

Gravity Loads include (1) Self Weight (2) Live Load (3) Snow Load Diaphragm & Joist Purlin & Kicker Beam & Post Roof & Floor Trusses From Top to Bottom through Continuous Load Path to Foundation to Bearing Soil Simple Mechanics

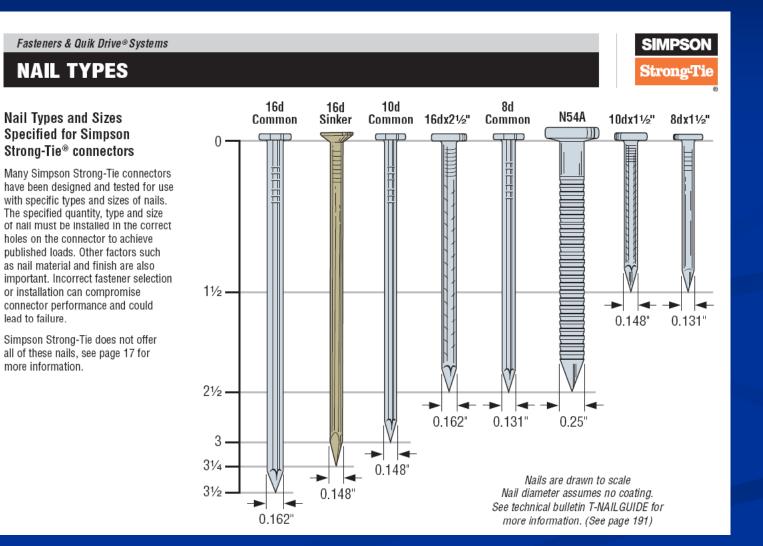


CONNECTORS

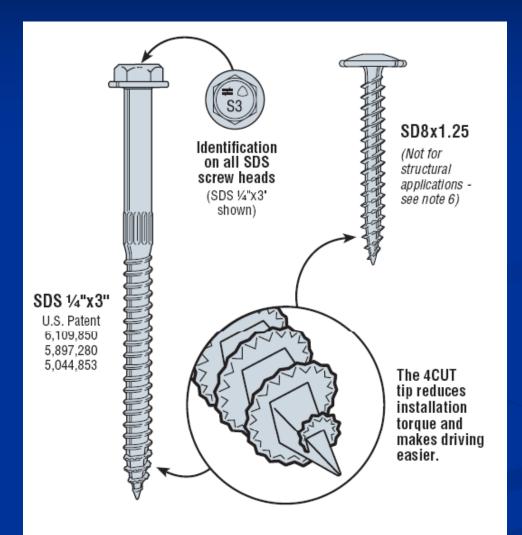
- Nails
- Lag Screws
- Anchor Bolts
- Hangers
- Metal Straps
- Hold-downs
- Beam to Post Seats
- Shear Transfer Clips
- Etc.



Nails





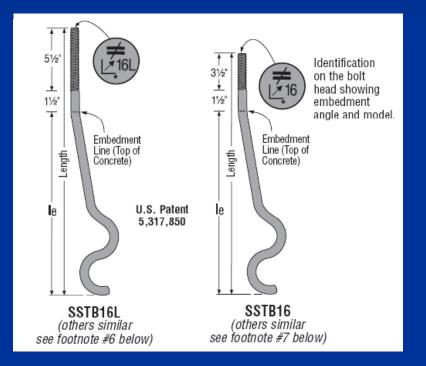


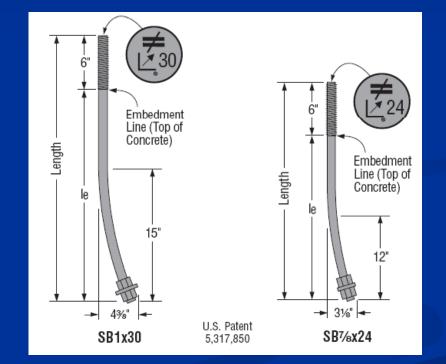


SDS and SD Wood Screws

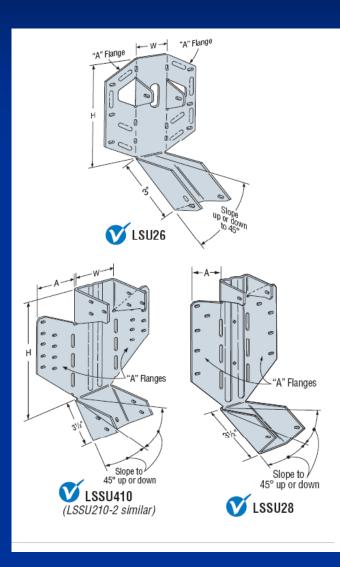
	Size (in.)	Model No.	Thread Length (in.)	Fasteners per Carton®	DF/SP Allowable Loads ⁴						SPF/HF Allowable Loads ⁴						
					Shear (100) ¹					Withdrawal⁵		Shear (100) Withdray			Withdrawal⁵	5	
					Wood Si	de Plate³	Steel Side Plate			(100)	Wood Side Plate ³		Steel Side Plate			(100)	Code Ref.
	. ,				1½"	1¾" SCL	16 ga	14 ga & 12 ga	10 ga or Greater	Wood or Steel Side Plate	1½"	1¾" SPF LVL	16 ga	14 ga & 12 ga	10 ga or Greater	Wood or Steel Side Plate	
	5⁄32 х 11⁄4	SD8x1.256,7	_	—	-	—	50	50	50	—	—	—	45	45	45	—	170
	1⁄4 x 11⁄2	SDS25112	1	1500	—	—	250	250	250	170	—		180	180	180	120	
	1⁄4 x 2	SDS25200	11⁄4	1300	_	_	_	290	290	215	—	—	—	210	210	150	
	1⁄4 x 21⁄2	SDS25212	11⁄2	1100	190	_	_	390	420	255	135		—	280	300	180	15,
	1⁄4 x 3	SDS25300	2	950	280	_	_	420	420	345	200	_	—	300	300	240	L17,
	1⁄4 x 31⁄2	SDS25312	21⁄4	900	340	340	—	420	420	385	245	245	—	300	300	270	F20
	1⁄4 x 41⁄2	SDS25412	2¾	800	350	340	—	420	420	475	250	245	—	300	300	330	
	1⁄4 x 6	SDS25600	31⁄4	600	350	340	_	420	420	560	250	245	—	300	300	395	

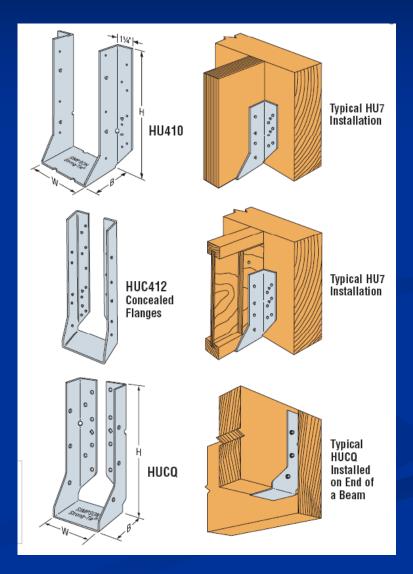
Anchor Bolts



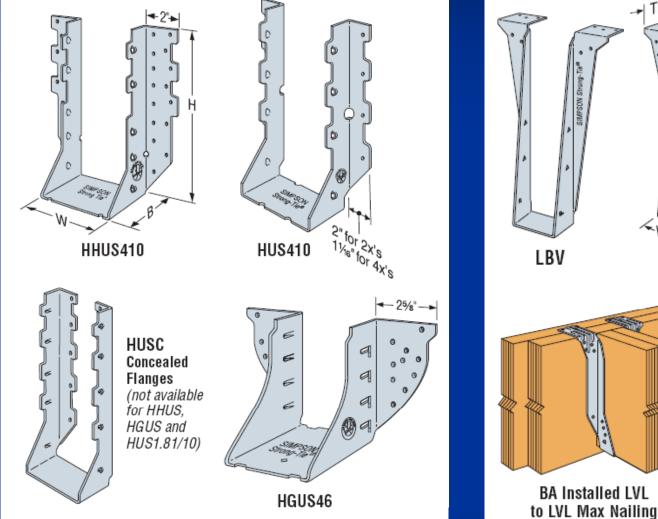


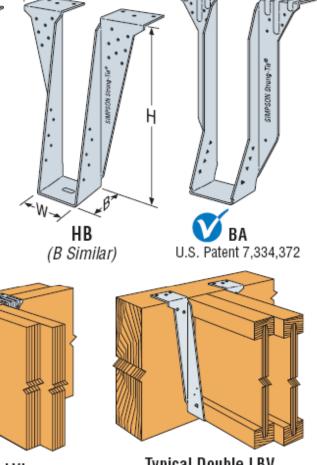
Hangers





Hangers

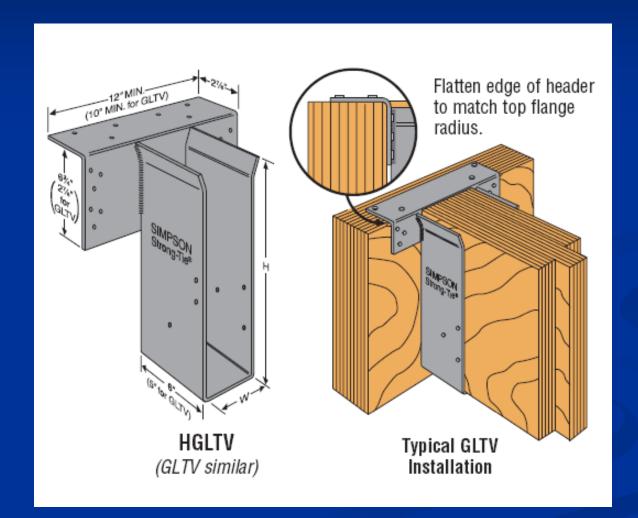




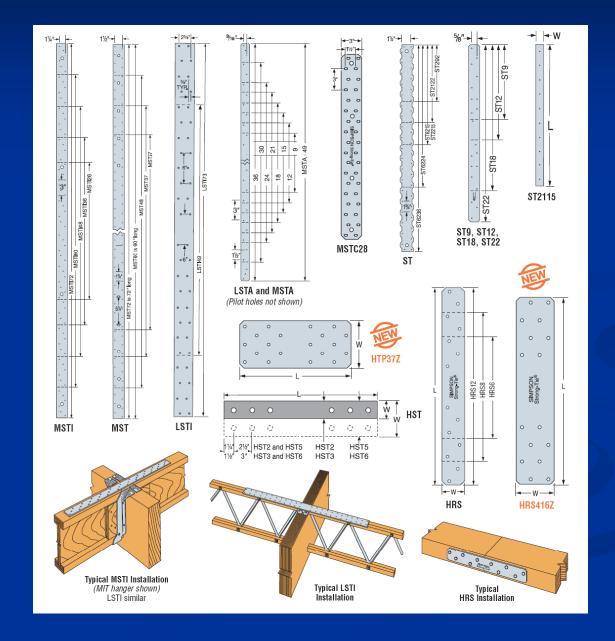
TF -

Typical Double LBV Hanger Installation

Hangers



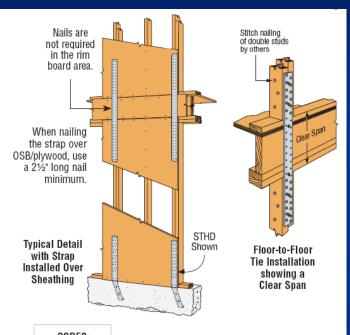
Metal Straps



Tie-Down Straps

Floor-to-Floor Clear Span Table

Model No.	Clear Span	Fasteners (Total)	Allowable Tension Loads (DF/SP)	Allowable Tension Loads (SPF/HF)		
	ohan	(Total)	(160)	(160)		
MSTA49	18	26-10d	2020	2020		
W51A49	16	26-10d	2020	2020		
MSTC28	18	12-16d sinkers	1155	995		
10131020	16	16-16d sinkers	1540	1325		
MSTC40	18	28-16d sinkers	2695	2320		
10131040	16	36-16d sinkers	3465	2980		
MSTC52	18	44-16d sinkers	4235	3645		
W61052	16	48-16d sinkers	4620	3975		
MSTC66	18	64-16d sinkers	5860	5495		
10131000	16	68-16d sinkers	5860	5840		
MSTC78	18	76-16d sinkers	5860	5860		
10131070	16	76-16d sinkers	5860	5860		
MST37	18	20-16d	2465	2135		
1013137	16	22-16d	2710	2345		
MST48	18	32-16d	3695	3425		
1013140	16	34-16d	3695	3640		
MST60	18	46-16d	4830	4830		
1013100	16	48-16d	4830	4830		
MST72	18	46-16d	4830	4830		
WI5172	16	48-16d	4830	4830		



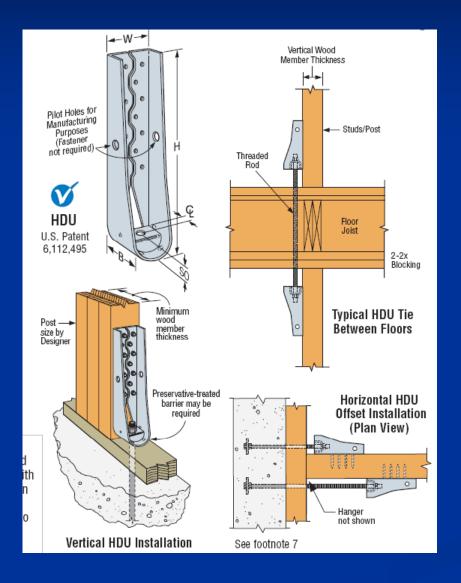
CODES: See page 12 for Code Reference Key Chart.

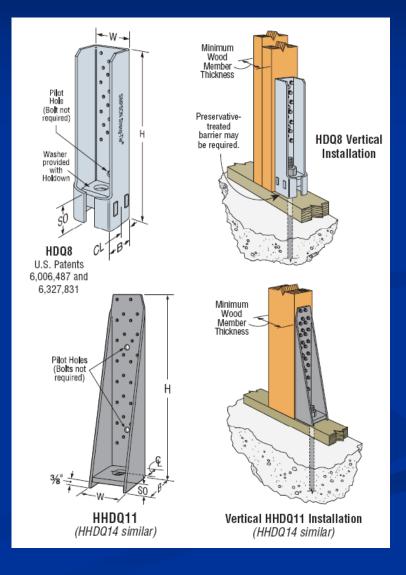
- Loads include a 60% load duration increase on the fasteners for wind or earthquake loading.
- 2. 10d commons may be substituted where 16d sinkers are specified at 100% of the table loads.
- 16d sinkers (0.148' dia. x 3¼' long) or 10d commons may be substituted where 16d commons are specified at 0.84 of the table loads.
- Allowable bolt loads are based on parallel-tograin loading and these minimum member thicknesses: MST-2½"; HST2 and HST5-4"; HST3 and HST6-4½".
- Use half of the required nalls in each member being connected to achieve the listed loads.
- Straps not installed over sheathing with 10d (0.148 dia. x 3) nails specified may be substituted with 10dx1½ (0.148 dia. x 1½) with no reduction in load.
- Tension loads apply for uplift as well when installed vertically.
- NAILS: 16d = 0.162" dia. x 3½" long, 16d Sinker = 0.148" dia. x 3½" long, 10dx1½ = 0.148' dia. x 1½" long. See page 16-17 for other nail sizes and information.

These products are available with additional corrosion protection. Additional produ	cts on
this page may also be available with this option, check with Simpson Strong-Tie for	r details.

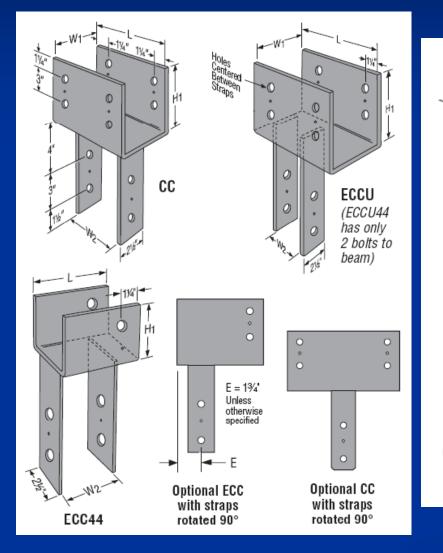
Model No.	Ga	Dimer	nsions	Fasteners (Total)				Tension Loads F/SP)	Allowable T (SP	Code	
		w	-	Nails	Bolts		Nails	Bolts	Nails	Bolts	Ref.
		VV	1	Nalis	Qty	Dia	(160)	(160)	(160)	(160)	
MST27		21⁄16	27	30-16d	4	1⁄2	3700	2165	3200	2000	
MST37	12	21⁄16	371⁄2	42-16d	6	1/2	5080	3025	4480	2805	
MST48		21⁄16	48	50-16d	8	1/2	5310	3675	5190	3410	
MST60	10	21⁄16	60	<mark>68</mark> -16d	10	1/2	6730	4485	6475	4175	
MST72	10	21⁄16	72	68-16d	10	1/2	6730	4485	6475	4175	14, L19, F2
HST2	7	21⁄2	211⁄4	—	6	5⁄8	—	5220	_	4835	L13,12
HST5	'	5	211⁄4	—	12	5⁄8	_	10650	_	9870	
HST3	3	3	251⁄2	—	6	3⁄4	—	7680	_	6660	
HST6	3	6	251⁄2	_	12	3⁄4	_	15470	_	13320	

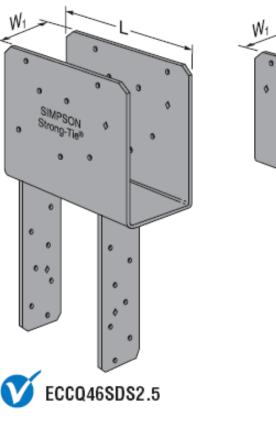
Hold-Downs

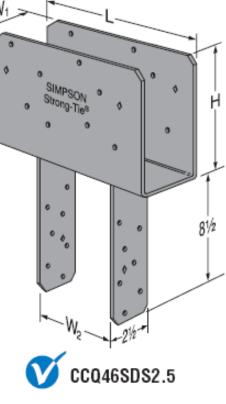




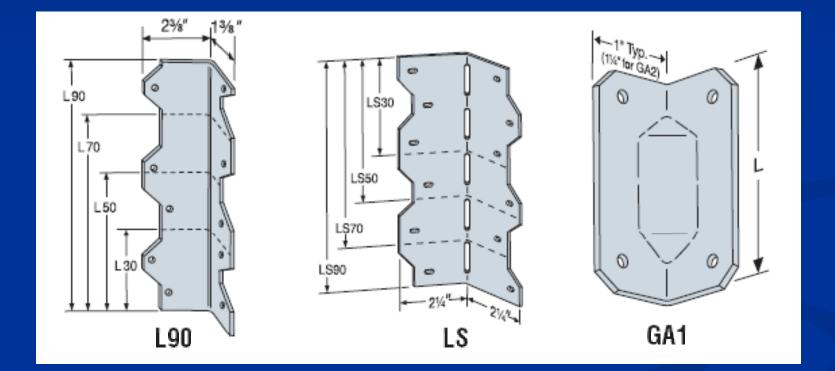
Beam To Post Seats



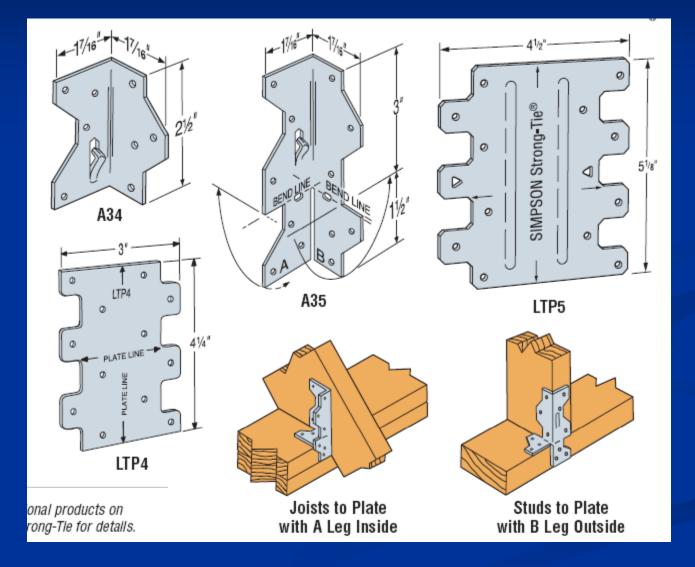




Shear Transfer Clips



Shear Transfer Clips



PRINCIPLES OF STRUCTURAL DESIGN IN WOOD

- Keep Simple, Symmetry, Uniform, Proportion, Module
- Watch out elevation difference vertically and horizontally for load path continuity
- Avoid big beam as possible unless necessary
- Avoid use Steel Beam
- Use nails and/or lag screws in stead of bolts as possible
- Consider construction & architectural aspects

WOOD DESIGN IN REAL LIFE

- Review the architectural drawings, civil drawings, and soil report
- Establish vertical and lateral loadings
- Layout the structural framing for vertical load
- Layout the lateral load resisting system
- Layout the foundation system
- Perform calculations and design
- Modify and revise and repeat as necessary

Real Design Case

- New Two Story Single Family Residence with Basement
- Location: Cupertino, California, USA
- Lot Condition: Empty lot with moderate hill slop

RESEARCH TOPICS

- New lateral force resisting elements
- New modular construction and material
- New analysis and design approach
- Innovative connection
- The application of Bamboo in Wood Structural Panel by Bamboo
 Stud & Beam by Bamboo

Any more???

Bored?? Interested?? Need help??

 Chia-Ching Lin (林家慶) (510)6230681
 <u>cclin 510@yahoo.com</u>
 cclin510@gmail.com